

EVALUATION OF SEEPAGE AND STABILITY OF DUHOK DAM

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ABSTRACT

One of the main causes of the earth dam failure is the seepage. This seepage can cause weakening in the earth dam structure, followed by a sudden failure due to piping or sloughing. For this purpose a finite element method through a computer program, named SEEP2D, was used to determine the free surface seepage line, the quantity of seepage through the dam, the pore water pressure distribution, the total head measurements and the effect of anisotropy of the core materials of Duhok zoned earth dam. First, the accuracy of the program was tested via the data of experimental dam and the results showed an acceptable accuracy of the program. The effect of the ratio of the permeability in the horizontal direction to that in the vertical direction (K_x/K_y) on seepage was tested and results indicated an increase in seepage quantity as this ratio increased. The stability of Duhok zoned earth dam was analyzed using a slope stability computer program, named STABIL2.3. The program is verified through a dam example of known factor of safety (solved by hand calculations). The results of the verification indicated a good accuracy of the program. The slope stability analysis results showed that the factor of safety decreases with the increase of K_x/K_y ratio. The analysis of the results of this study showed that Duhok zoned earth dam is safe against the danger of piping and slope sloughing under the present operation levels. Also, the present study showed that the field piezometers readings of the dam are not accurate.

Keywords: Seepage, Stability, Duhok dam

تقييم التسرب والاستقرارية لسد دهوك

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الخلاصة

ان التسرب خلال السدود الترابية من العوامل الرئيسية التي تؤدي الى انهيار هذه السدود نتيجة لحدوث التسرب التآكلي أو الانسلاخ. لذلك تم تطبيق طريقة العناصر المحددة باستخدام برنامج كومبيوتر جاهز باسم (SEEP2D) على سد دهوك الترابي غير المتجانس (zearth dam oned) لحساب السطح الحر وكمية التسرب خلال جسم السد وتوزيع ضغط الماء المسامي وقراءة الشحنة الكلية وتأثير المواد غير موحدة الخواص لللب السد. في البداية تم اختبار دقة البرنامج وذلك بادخال بيانات نموذج سد مختبري ومقارنة النتائج المستحصلة من البرنامج المذكور مع النتائج المختبرية المتوفرة وكانت نتائج الاختبار مقبولة. كما تم دراسة تأثير نسبة معامل التسرب بالاتجاه الافقي الى معامل التسرب بالاتجاه العمودي (K_x/K_y) على التسرب وتبين بان زيادة النسبة (K_x/K_y) يؤدي الى رفع السطح الحر وزيادة كمية التصريف المتسرب من خلال السد. بالإضافة الى ذلك تم استخدام برنامج كومبيوتر جاهز باسم (STABIL2.3) لدراسة استقرارية ميل السد واختبار دقة البرنامج بأخذ سد بمعامل امان معلوم يمكن حسابه عن طريق الحل اليدوي وكانت النتائج جيدة ومقبولة. كما بينت نتائج تحليل الاستقرارية بان عامل الامان يقل بزيادة النسبة (K_x/K_y). لقد أظهرت نتائج هذه الدراسة أن سد دهوك الترابي غير المتجانس مستقر وغير معرض الى خطر الفشل الناجم عن التسرب واستقرارية الميل معاضن المناسب التشغيلية في الوقت الحاضر.

الكلمات الدالة: تسرب، استقرارية، سد دهوك

1. INTRODUCTION

Duhok dam constructed in 1987 is located on Rubar Duhok river 2km to the north of Duhok city center. Both main and coffer dams are earth fill type with central clay core followed by one layer of filter on both sides and supported by sand and gravel shell as shown in Fig. (1). One of the important stages in design of earth and rock-fill dam is the exact evaluation of seepage quantity, pore water pressure distribution used in the analysis of slope stability, total head measurements and hydraulic gradients in various sections of the dam to ensure stability and avoid endangering effects such as piping and slope sloughing [1].

The main objectives of this study are:

1. Theoretical seepage analysis for Duhok zoned earth dam using the finite element method through a computer program named (SEEP2D) at different sections.
2. Comparing the theoretical results with the field piezometer readings.
3. Investigating the effect of anisotropy of core materials.
4. Analyzing the slope stability of different dam sections through a computer program named (STABIL2.3) performing the conventional limit equilibrium method of slices, the simplified Bishop method and Fellenius method.

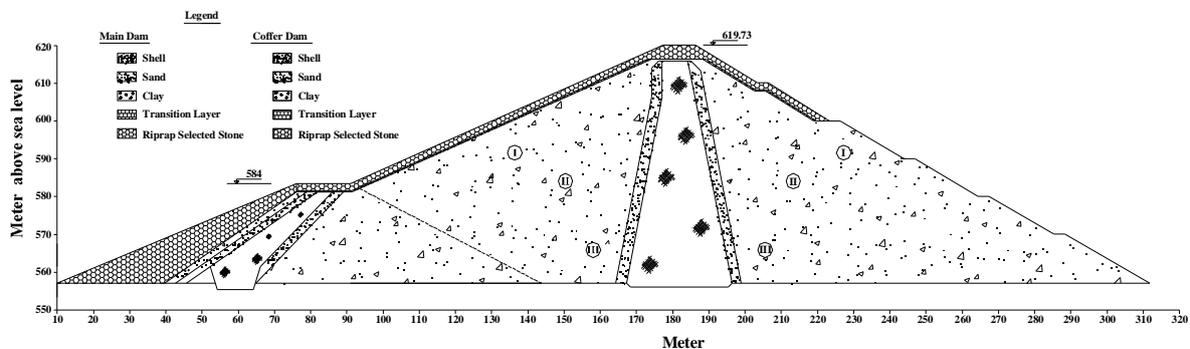


Fig. (1) Typical cross section of Duhok zoned earth dam

2. SEEPAGE FLOW THROUGH EARTH DAMS

Seepage flow of water through porous media depends on the soil media, type of flow, properties of liquid and hydraulic gradient. Seepage piping account for approximately 50% of all earth dam failures [2]. Many methods have been developed to solve seepage problems, these methods can be classified as analytical, experimental and approximate methods [3,4,5,6].

Mishra and Sing [7] analyzed a steady state 2D flow through a homogeneous levee with a horizontal toe drain resting on an impervious base using fragments method. Dupuit, Schaffernak, Pavlovsky, Cassagrande, and Polubarinova-kochina, developed approximate methods to calculate the seepage quantity and the free surface seepage line of earth dams [3, 4, 5]. *Billstein et al.* [8] used experimental models to determine discharge, pore water pressure, seepage face and free surface profile. Most of mentioned methods are valid for simple geometries, isotropy and homogeneous soil media, but with complex geometry, anisotropy, non homogeneous and nonlinear material behavior these methods are difficult to be used and may limit the solution accuracy. Therefore, during the last 30 years, considerable advances have been made in the theory and practice with the help of high speed digital computers. As a result, the numerical methods (finite element, finite difference and boundary element) and their applications are widely used for the solution of many engineering problems. The finite element method (FEM) is one of these numerical methods that can be used for accurate solution of many complex engineering problems. Over the years, the FEM seems to be the most powerful and versatile tool over other numerical methods for solving a

wide variety of practical problems efficiently [9]. *Muhammad* [10] used the finite element method to study seepage through earth dams. *Abo* [11] used the FEM to determine the free surface position, the seepage quantity, the pore water pressure distribution, the total head measurements and the anisotropy of the material in AL-Adheem zoned earth dam. *Thieu et al.* [12] used 2-D and 3-D finite element method for modeling the seepage in a saturated/unsaturated soil system of the earth dams under steady state and transient seepage conditions. *Bardet and Tobita* [13] used a finite difference method for the solution of unconfined seepage with an unknown free surface.

3. SLOPE STABILITY

Every soil mass which has a slope at its end is subjected to shear stresses on internal surfaces or failure plans in the soil mass near the slope. This is due to the gravitational forces that try to pull down parts of the soil mass adjoining the slope. Several models and analytical techniques have been developed to determine the critical slip surface and the associated factor of safety such as method of slices. The factor of safety is:

$$FS = \frac{\tau_f}{\tau} \quad (1)$$

where, FS = factor of safety,

τ_f = failure shear strength of the soil, and

τ = shear stress of the soil.

Fellenius (1936) (cited in [14]) developed the Ordinary Method of Slices (OMS) known as Felonious or Swedish method. It is assumed that the forces acting on sides of any slice are neglected. *Bishop* [15] developed a method based on the assumption that the interslice forces are horizontal. This method is called the Simplified Bishop Method (SBM) in which a circle slip surface is assumed. *Janbu* (1956) (cited in [16]) developed a simplified method that assumes zero interstice shear forces. *Morgenstern and Price* (1965) (cited in [17]) developed a general analysis in which all boundary and equilibrium conditions were satisfied and in which the failure surface might be of any shape, circular, non-circular or compound. *Wright et al.* [18] used the finite element method to determine the factor of safety and associated critical slip surface and compared with the simplified Bishop method. *Martins et al.* [19] used the rigid-plastic methods (Felonious and Bishop) and the elasto-plastic finite element method (FEM) to analyze the slope stability of embankment with and without surcharge. *Hammah et al.* [20] compared the finite element method with the limit equilibrium methods for slope stability analysis.

4. SEEPAGE THEORY

The governing partial differential flow equation for steady state condition two dimensional flow through anisotropic, homogeneous porous media is presented by Laplace equation as:

$$\frac{\partial}{\partial x} \left(K_x \frac{\partial H}{\partial x} \right) + \frac{\partial}{\partial y} \left(K_y \frac{\partial H}{\partial y} \right) = 0 \quad (2)$$

where, K_x, K_y = coefficient of permeability in (x,y) directions,

H = total head of water and equal to $\frac{p}{\gamma_w} + z$,

p = pore water pressure,

γ_w = unit weight of water, and

z = elevation head above sea level (a. s. l.).

For unconfined seepage problems there are four boundary conditions [3, 5, 6] :

1. **Impervious boundary**

$$\frac{\partial H}{\partial n} = 0 \quad (3)$$

where, n = vertical direction of the boundary.

2. **Entrances and exits:** also called reservoir boundaries or submerged permeable boundaries

$$H = h_1 \quad (4)$$

$$H = h_2 \quad (5)$$

where, h_1 and h_2 are heads of water at entrance and exit, respectively.

3. **Surface of seepage**

$$H = y \quad (6)$$

4. **Line of seepage**

$$H = y \quad (7)$$

$$\frac{\partial H}{\partial n} = 0 \quad (8)$$

5. FINITE ELEMENT METHOD

The basic concept of the finite element method is to divide the problem region into subdomains (finite elements) connected at their common nodal points and that the unknown function of the field variable is defined approximately within each element. The approximate solution of each element expressed by continuous function is as follows:

$$\bar{H}^e = \sum_{i=1}^I N_i^e H_i^e \quad (9)$$

where, \bar{H}^e = approximate solution for element (e),

H_i^e = nodal value of (H) for i node in element (e),

I = No. of nodes in element (e), and

N_i^e = shape function of element (e).

In matrix form, Eq. (9) can be formulated as:

$$\bar{H}^e = [N_i^e] \{H_i^e\} \quad (10)$$

where, $\{H_i^e\}$ = matrix of nodal values of domain variables, and

$[N_i^e]$ = matrix of shape functions.

The approximate solution of the variable (H) in the global domain of the problem is expressed as follows:

$$\bar{H} = \sum_{e=1}^{noe} [N_i^e] \{H_i^e\} \quad (11)$$

where, noe = total number of elements.

There are more different approaches to formulate the approximate solution of the problems. In the present investigation, the weighted residual method with Galerkin's criterion is used:

$$\int_v W_j R \, dv = 0 \quad (12)$$

where, R = the element residual, and

W_j = the weighting function.

Various forms of weighting function sets can be used in practice, each leading to a different weighted residual approximation method. One of the best approximations is called the Galerkin's method, in which the weighting function is made equal to the shape function defining the approximation. Applying this principle in finite element method over the whole problem domain, Eq. (12) can be written as:

$$\sum_{e=1}^{noe} \int_{v^e} N_j^e F(\bar{H}^e) \, dv = 0 \quad (13)$$

where, v^e = the domain of element (e).

Substituting Eq. (9) into Eq. (2) yields:

$$\frac{\partial}{\partial x} (K_x \frac{\partial}{\partial x} \sum_{i=1}^I N_i H_i) + \frac{\partial}{\partial y} (K_y \frac{\partial}{\partial y} \sum_{i=1}^I N_i H_i) = R = 0 \quad (14)$$

Applying Galerkin's principle, and substituting Eq. (14) into Eq. (13) yields:

$$\sum_{e=1}^{noe} \int_{v^e} N_j^e [\frac{\partial}{\partial x} (K_x \frac{\partial}{\partial x} \sum_{i=1}^I N_i H_i) + \frac{\partial}{\partial y} (K_y \frac{\partial}{\partial y} \sum_{i=1}^I N_i H_i)] \, dv = 0 \quad (15)$$

where, $dv = dx \, dy \, dz$,

$j = 1$ to I , and

I = number of element nodes.

Integrating Eq. (15) by parts and applying Green-Gauss theorem yields:

$$\sum_{e=1}^{noe} \int_v [\int -(\frac{\partial N_j}{\partial x} K_x \frac{\partial}{\partial x} \sum_{i=1}^I N_i H_i + \frac{\partial N_j}{\partial y} K_y \frac{\partial}{\partial y} \sum_{i=1}^I N_i H_i) \, dv + \int_s N_j K_n \frac{\partial}{\partial n} \sum_{i=1}^I N_i H_i \, ds] = 0 \quad (16)$$

where, K_n = coefficient of permeability normal to the surface boundaries of the element,

s = surface boundaries of the element

Now defining $H = H_o$ as reservoir boundary condition and $v_n = 0$ as impervious boundary condition then,

$$v_x L_x + v_y L_y = v_n = 0$$

$$K_x \frac{\partial H}{\partial x} L_x + K_y \frac{\partial H}{\partial y} L_y = 0$$

Applying the above conditions into Eq. (9) yields:

$$K_x \frac{\partial}{\partial x} \sum_{i=1}^I N_i H_i L_x + K_y \frac{\partial}{\partial y} \sum_{i=1}^I N_i H_i L_y = R = 0 \quad (17)$$

Using weighted residual Galerkin's method, Eq. (17) can be written as:

$$\sum_{e=1}^{noe} \int_{v^e} \left[\left(\frac{\partial N_j}{\partial x} K_x \frac{\partial}{\partial x} \sum_{i=1}^I N_i H_i + \frac{\partial N_j}{\partial y} K_y \frac{\partial}{\partial y} \sum_{i=1}^I N_i H_i \right) dx dy dz - \int_{s_i^e} (N_j K_x \frac{\partial}{\partial x} \sum_{i=1}^I N_i H_i L_x + N_j K_y \frac{\partial}{\partial y} \sum_{i=1}^I N_i H_i L_y) ds \right] = 0 \quad (18)$$

where, s_i^e = surface boundaries of element i,

$$dx = L_x ds, \text{ and}$$

$$dy = L_y ds.$$

And in matrix form Eq. (18) can be written as:

$$\sum_{e=1}^{noe} [k^e] [H] = 0 \quad (19)$$

where, $[k^e]$ represents the element stiffness matrix and known as follows:

$$k_{ij} = \int_{v^e} \left(\frac{\partial N_j}{\partial x} K_x \frac{\partial N_i}{\partial x} + \frac{\partial N_j}{\partial y} K_y \frac{\partial N_i}{\partial y} \right) dx dy - \int_{s_i^e} (K_x L_x N_i \frac{\partial N_j}{\partial x} + K_y L_y N_i \frac{\partial N_j}{\partial y}) ds \quad (20)$$

Surface integral s_i^e in Eq. (20) is unknown. This condition doesn't cause any problem in the solution of the differential equation because it is not related to the nodes that exist on the free surface but to the reservoir boundary.

Eq. (20) doesn't apply in this part because the head is known; therefore, the element matrix can be simplified as:

$$k_{ij} = \int_{v^e} \left(\frac{\partial N_j}{\partial x} K_x \frac{\partial N_i}{\partial x} + \frac{\partial N_j}{\partial y} K_y \frac{\partial N_i}{\partial y} \right) dx dy \quad (21)$$

where, N_i = interpolation or shape function.

The interpolation function for triangular element can be expressed as:

$$N_i = a_i + b_i x + c_i y \quad (22)$$

For quadrilateral element

$$N_i = a_i + b_i x + c_i y + d_i xy \quad (23)$$

Derivations of the above functions are shown in [21].

6. SLOPE STABILITY ANALYSIS

In the present study, two traditional methods for slope stability analysis of Duhok zoned earth dam are used. These methods are:

6.1 Fellenius (Swedish) Method

For steady state seepage condition the factor of safety using Fellenius method is:

$$FS = \frac{\sum_{i=1}^n (c' L_i + (W_i \cos \alpha_i - u_i L_i) \tan \phi')}{\sum_{i=1}^n W_i \sin \alpha_i} \quad (24)$$

6.2 Simplified Bishop Method

For steady state seepage condition the factor of safety using Bishop method is:

$$FS = \frac{\sum_{i=1}^n (c' b_i + (W_i - u_i b_i) \tan \phi') \frac{1}{m_{\alpha i}}}{\sum_{i=1}^n W_i \sin \alpha_i} \quad (25)$$

$$\text{Where, } m_{\alpha i} = \cos \alpha_i + \frac{\tan \phi \sin \alpha_i}{FS} \quad (26)$$

- c' = effective soil cohesion,
- L = length of the bottom of the slice,
- b = width of the slice and equal to $(L \cos \alpha)$,
- u = pore water pressure,
- W = weight of the slice,
- α = inclination of the bottom of the slice, and
- ϕ' = effective internal friction angle.

7. RESULTS AND DISCUSSIONS

The results obtained by the application of the finite element program, named (SEEP2D), to Duhok zoned earth dam are analyzed and discussed extensively. The results include the location of the free surface seepage line, the quantity of seepage through the dam, the pore water pressure distribution, the total head measurements and the effect of anisotropy of the core material. For the verification of the program, experimental data of homogeneous dam constructed by Kellogg (cited in Abo [11]) where used. Kellogg's dam was built with coarse sand of 38 cm height and 68 cm base length with upstream and downstream slopes of 1H:1.3V. The origin of the x and y coordinates of the points tested was located at upstream slope toe, for more details, see [21]. The results indicated an acceptable accuracy of the program as shown in Table (1).

The results of the slope stability analysis are also presented and discussed using a computer program, named (STABIL2.3), which used the conventional slope stability methods of slices (simplified Bishop and Fellenius methods) for determining the minimum values of the factor of safety and the associated critical slip surfaces. The program is verified using three dam examples of known factors of safety (solved by hand calculations), for more details, see [21]. The results of the verification indicated a good accuracy of the program as shown in Table (2). The seepage and slope stability analysis of Duhok zoned earth dam shown in Fig. (1) is studied according to the material properties illustrated in Table (3) [22].

The seepage through Duhok earth dam, which is a non-homogeneous earth dam (zoned earth dam) consisting of the earth fill shell with central clay core is studied. According to "SEEP2D Primer" [23] a portion of downstream is omitted because the ratio of permeability between the shell and the core material is more than one hundred so as to avoid any confusion in the solution.

Table (1): Comparison of total head measurements for verification

Piezometer No.	Coordinates (cm)		Total head (cm)	Total head (cm)	Tolerance %
	x	y	Experiment*	F. E. M.	
1	7.86	3.93	37.34	37.48	0.37
2	31.26	14.41	33.02	31.42	4.85
3	31.32	5.06	32.00	30.20	5.63
4	35.03	18.74	31.50	30.53	3.08
5	47.75	5.16	21.84	20.56	5.86
6	61.81	5.90	9.15	8.75	4.37
7	44.55	13.10	24.13	23.56	2.36

* after Kellogg(cited in Abo [11])

Table (2): Comparison of minimum factor of safety for verification

		Dam example (1)	Dam example (2)	Dam example (3)
Hand solution		2.44 *	2.71	2.3
Program results	Fellenius	2.836	2.387	2.084
	Simplified Bishop	2.862	2.689	2.227

* Calculated with tension crack = 2.14 m

Table (3): Properties of Duhok zoned earth dam materials [22]

No.	Physical and mechanical properties	Unit								Foundati on ballast
			Stone	Ballast			Sand	Clay		
				Zone I	Zone II	Zone III		During operation	During construct ion	
1	Dry Density- γ_d	gr/cm ³	1.70	2.00	2.00	2.00	2.00	1.56	1.56	1.95
2	Bulk Density Natural- γ_n	gr/cm ³	1.80	2.10	2.10	2.10	2.10	1.92	1.92	2.05
3	Saturated- γ_{sat}	gr/cm ³	2.15	2.20	2.20	2.20	2.20	1.99	1.99	2.15
4	Submerged- γ^-	gr/cm ³	1.15	1.20	1.20	1.20	1.20	0.99	0.99	1.15
5	Angle of internal friction- Φ	degree	35.50	43.50	39.00	37.00	37.00	15.23	11.00	30.00
6	Cohesion-c	kg/cm ²	0.10	0.00	0.00	0.00	0.00	0.30	0.42	0.05
7	Permeability-k	cm/sec		4.1×10^{-3}	4.1×10^{-3}	4.1×10^{-3}	4.1×10^{-3}	3.2×10^{-7}	3.2×10^{-7}	4.1×10^{-3}

7.1 Application of the (SEEP 2D) Program on Duhok Dam

For the seepage analysis of Duhok zoned earth dam, four sections are studied using a finite element computer program (SEEP2D) they are: sections (P1-P2, P3-P4, P5-P6 and P7-P8) and for different reservoir water levels (613.12, 611.16, 610.56 and 609.96 m, a. s. l.) [21]. These sections are located at piezometer stations shown in Fig. (2). In this paper the results of section P1-P2 are illustrated and discussed. The results of sections (P3-P4, P5-P6 and P7-P8) are shown else where see [21]. Section P1-P2 is located at station (150) m from

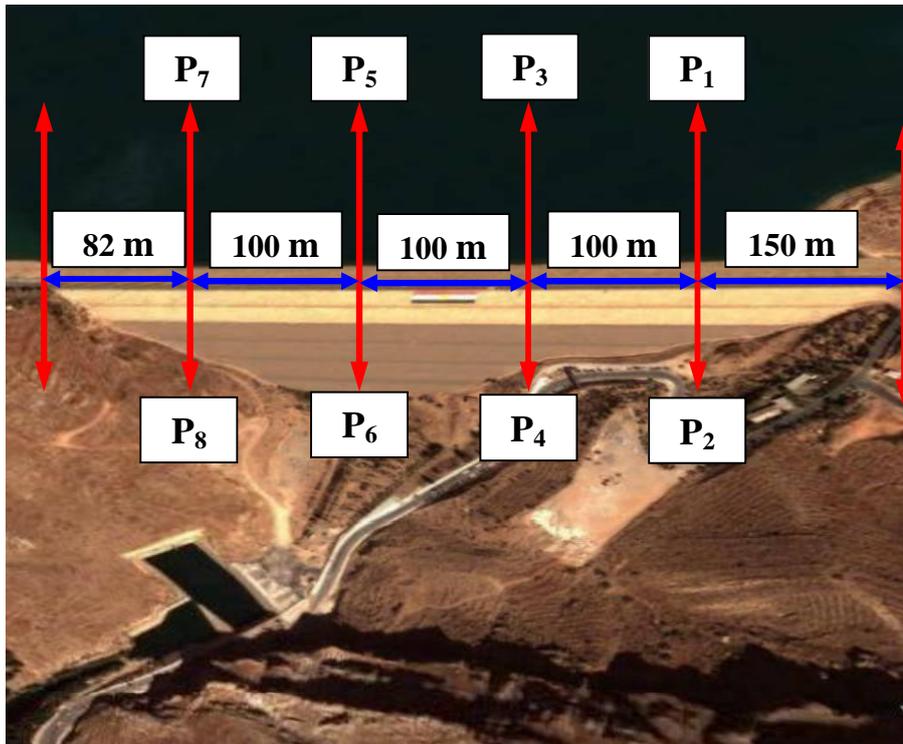


Fig. (2) Locations of piezometers

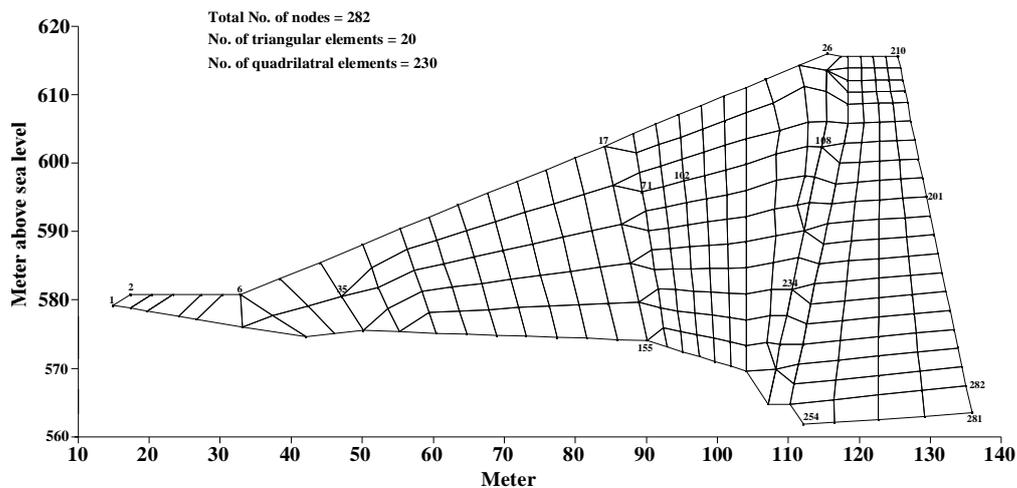


Fig. (3) Mesh construction of section P1-P2

the beginning of the dam along the crest length. Fig. (3) illustrates the construction of the mesh for this section. Table (4) represents a comparison of the finite element readings with field piezometer readings and showed that the finite element readings are greater than the field readings. This means that the phreatic line determined by the program is higher than the field phreatic line. So as to reach the field phreatic line the K_x/K_y ratio must be changed. For this purpose the K_x/K_y ratios are taken as (0.1, 0.2, 1, 5, and 10). The results indicate that the phreatic line is higher by increasing K_x/K_y ratio and is lower by decreasing it which agrees with the results given by [6, 10, 11]. When K_x/K_y increases the horizontal velocity is higher than the vertical velocity, therefore, the phreatic line will be more flat. To lower the phreatic line, K_x/K_y ratio should be less than 1, and this means that the coefficient of permeability in the vertical direction (K_y) must be higher than the coefficient of permeability in the horizontal direction (K_x), but this condition is not correct as the case in the field. Since, in the compaction process the soil is laid in horizontal layers and then compacted, therefore, K_x value must be higher than K_y value. Low field phreatic line may be attributed to one or more of the following reasons: (1) Inaccurate field piezometer readings. (2) Bad installation of the piezometers because they were installed after the dam construction. (3) Effect of consolidation of material. (4) Cracks or deformations in the dam. However there is no field evidence on that. Seepage quantity of section P1-P2 is shown in Table (5), while the total seepage quantity of the whole dam is shown in Table (6) for K_x/K_y values of (1, 5 and 10).

Table (4): Comparison between field piezometer and finite element readings for section P1- P2.

Reservoir water level (a. s. l.) (m)	Date	Piezometer	Field piezometer reading (m)	FEM reading (m)	Tolerance %
				$K_x/K_y = 1$	
613.12	27-7-2004	P1	29.00	28.88	0.41
		P2	17.69	22.60	21.73
611.16	14-10-2004	P1	27.02	26.92	0.44
		P2	17.27	22.60	23.58
610.51	18-11-2004	P1	26.39	26.29	0.38
		P2	17.15	22.60	24.12
609.96	26-1-2005	P1	25.68	25.72	0.16
		P2	16.89	22.60	25.27

Table (5): Seepage quantity (m^3/day) for section P1-P2.

K_x/K_y ratio	Reservoir water level, a. s. l. (m)			
	613.12	611.16	610.51	609.96
1	285.6	231.6	228.4	207.6
5	1355.3	1223.3	1176.5	1131.4
10	2760.3	2472.3	2377.8	2318.3

Table (6): Total seepage quantity (m³/day) for Duhok dam.

Kx/Ky ratio	Reservoir water level, a. s. l. (m)			
	613.12	611.16	610.51	609.96
1	1122.0	1046.0	1031.5	987.7
5	4663.5	4277.8	4153.5	4066.8
10	9166.5	8297.7	8078.7	7901.7

Figs (4) to (13) show the pore water pressure distribution and total head measurements for Kx/Ky = 0.1, 0.2, 1, 5 and 10 ratios and reservoir water level 613.12m (a. s. l.). Figs (14) and (15) represent the relationship between seepage quantity and Kx/Ky ratio and indicate that the seepage quantity increases with the increase of Kx/Ky ratio.

7.2 Application of the (STABIL2.3) Program on Duhok Dam

According to the seepage analysis of the four different cross sections of Duhok zoned earth dam, the slope stability of these sections are also studied using the computer program (STABIL2.3) [21], but in this paper the results of section P1-P2 are presented only.

The downstream slope stability of this section of the dam is studied using the computer program to determine the minimum factor of safety and associated critical slip surface. Two conditions are considered for the slope stability analysis, with and without seepage. In case of seepage consideration the steady state seepage analysis is used. So as to study the effect of the free surface line on stability of slopes, the (0.1, 1 and 10) Kx/Ky ratios are used for reservoir water level 613.12m (a. s. l.). According to the properties of materials of the dam in Table (3), the program is operated. Tables (7) and (8) show the results of stability analysis of this section while, Figs. (16) and (17) show the critical slip surfaces determined. The results indicated that this section is safe against slope failure in both cases (with and without seepage). The results also indicate that the factor of safety decreases with the increase of Kx/Ky ratio, this is due to the increase of pore water pressure and the free surface line will be higher with the increase of the Kx/Ky ratio, this agrees with the results concluded by [24].

Table (7): Minimum factor of safety for section P1-P2 (without seepage)

Method	F.S	Radius (m)
Fellenius	2.745	75.16
Simplified Bishop	2.700	75.16

Table (8): Minimum factor of safety for section P1-P2 (with seepage)

Kx/Ky	Fellenius		Simplified Bishop	
	F.S.	Radius (m)	F.S.	Radius (m)
0.1	1.534	68.00	1.578	68.00
1	1.500	68.00	1.544	68.00
10	1.493	68.00	1.538	68.00

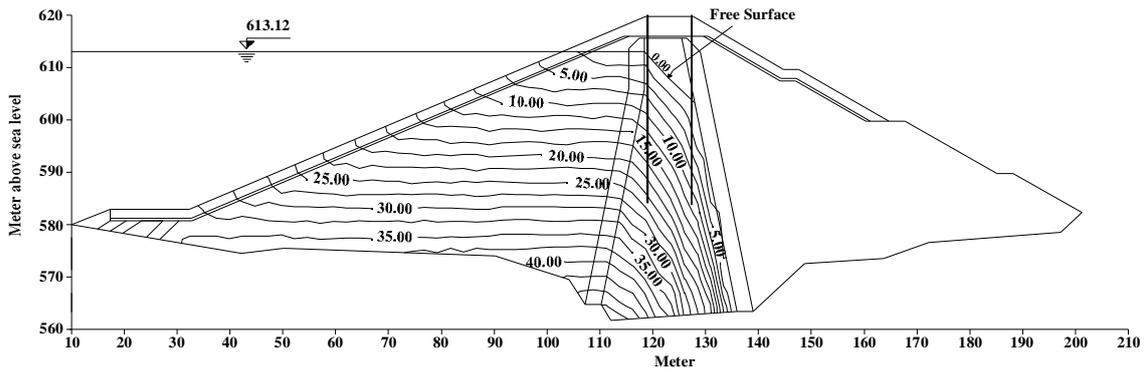


Fig. (4) Pore water pressure readings for reservoir water level 613.12 m , $K_x/K_y = 0.1$ and section P1-P2.

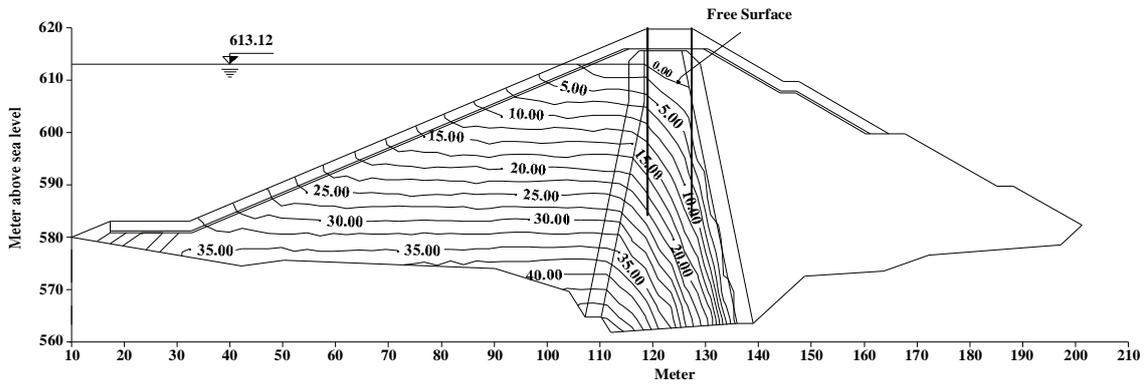


Fig. (5) Pore water pressure readings for reservoir water level 613.12 m , $K_x/K_y = 0.2$ and section P1-P2.

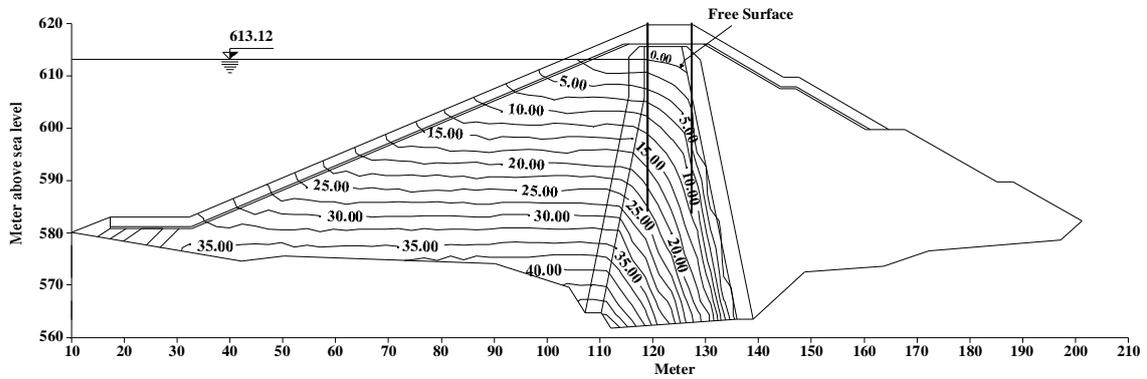


Fig. (6) Pore water pressure readings for reservoir water level 613.12 m , $K_x/K_y = 1$ and section P1-P2.

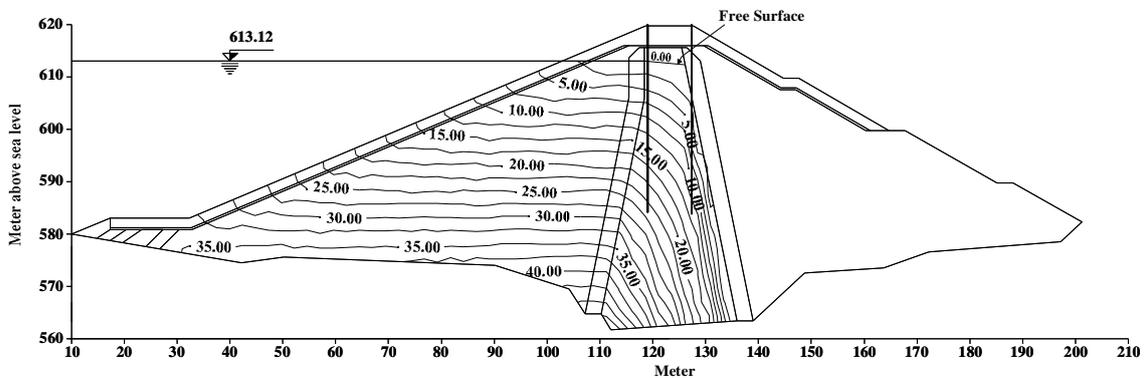


Fig. (7) Pore water pressure readings for reservoir water level 613.12 m , $K_x/K_y = 5$ and section P1-P2.

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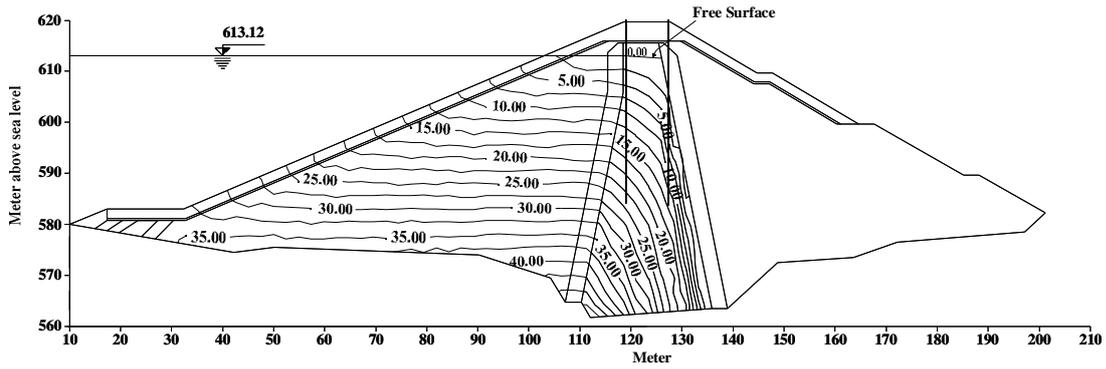


Fig. (8) Pore water pressure readings for reservoir water level 613.12 m , $K_x/K_y = 10$ and section P1-P2.

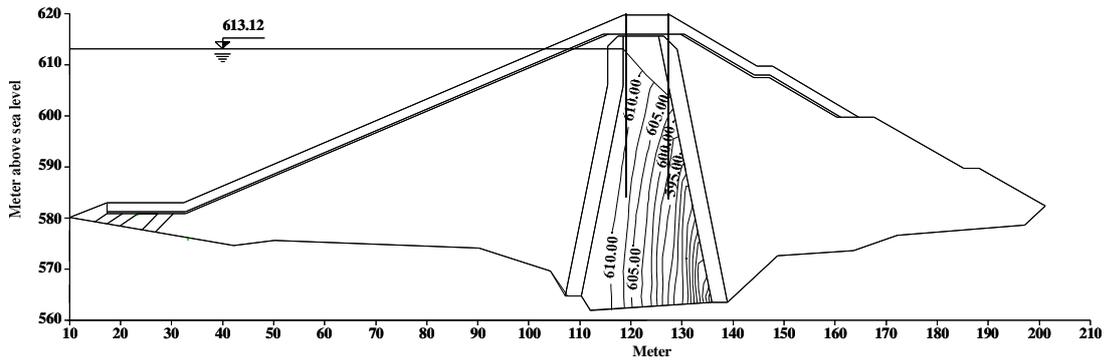


Fig. (9) Total Head measurements for reservoir water level 613.12 m, $K_x/K_y = 0.1$ and section P1-P2.

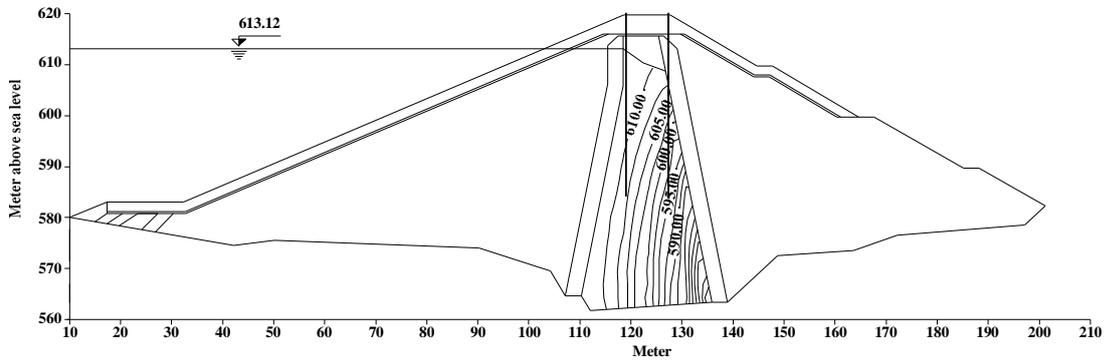


Fig. (10) Total Head measurements for reservoir water level 613.12 m, $K_x/K_y = 0.2$ and section P1-P2.

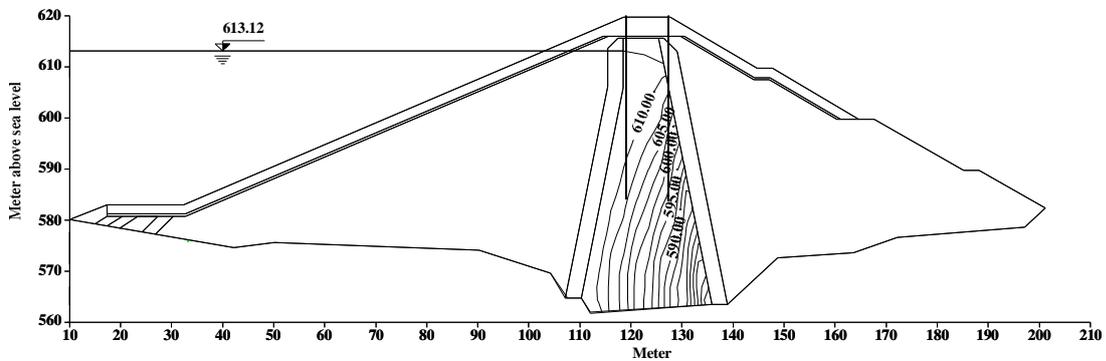


Fig. (11) Total Head measurements for reservoir water level 613.12 m, $K_x/K_y = 1$ and section P1-P2.

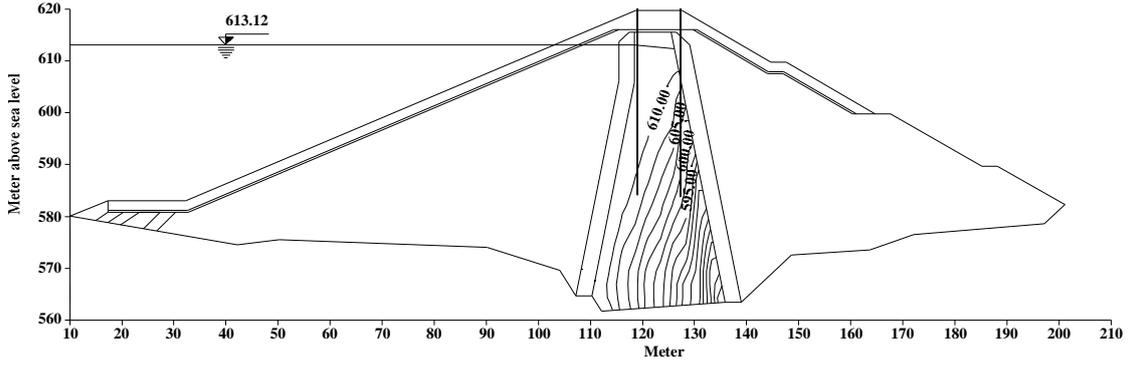


Fig. (12) Total Head measurements for reservoir water level 613.12 m, $K_x/K_y = 5$ and section P1-P2.

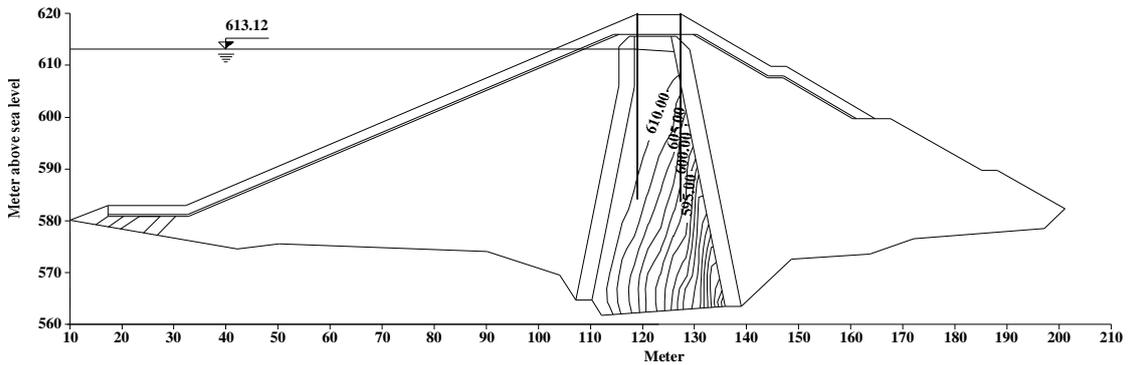


Fig. (13) Total Head measurements for reservoir water level 613.12 m, $K_x/K_y = 10$ and section P1-P2.

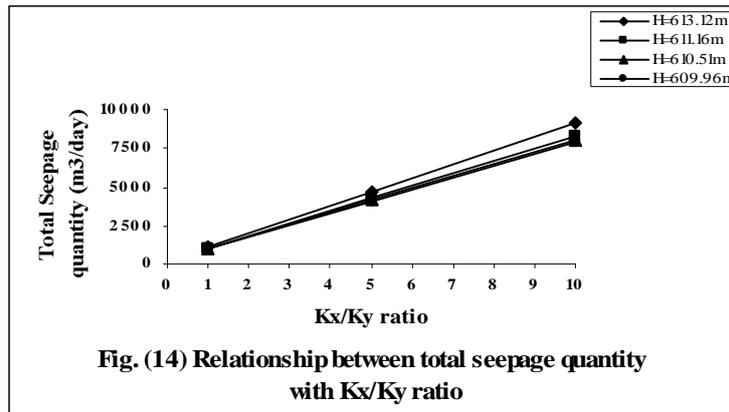


Fig. (14) Relationship between total seepage quantity with K_x/K_y ratio

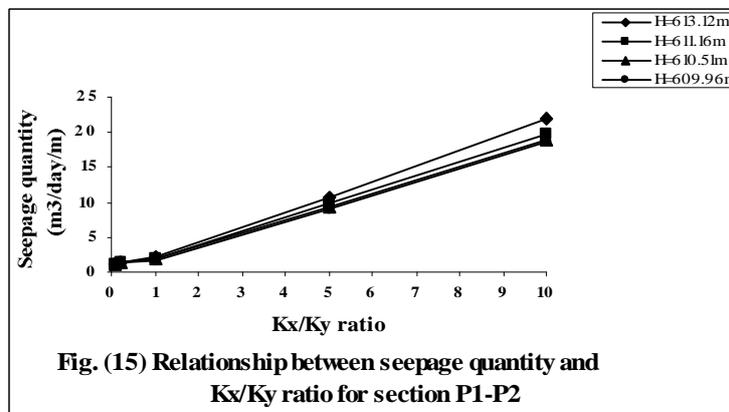


Fig. (15) Relationship between seepage quantity and K_x/K_y ratio for section P1-P2

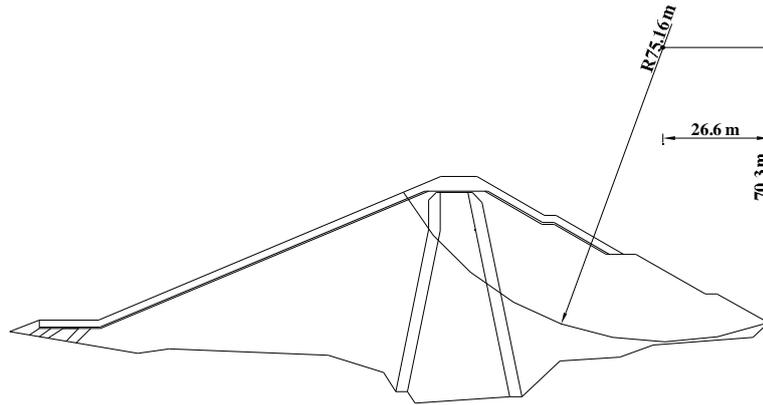


Fig. (16) Critical slip circle without seepage for section P1-P2 (Both methods).

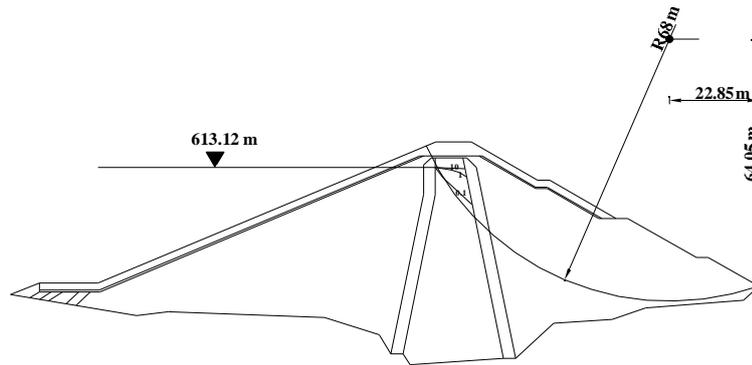


Fig. (17) Critical slip circle with seepage for section P1-P2 and $K_x/K_y = 0.1, 1$ and 10 (both methods).

8. CONCLUSIONS

1. The finite element program (SEEP2D) can be used to analyze the homogeneous and non-homogeneous (zoned) earth dams. Therefore, the program is applied on four different sections of the Duhok zoned earth dam so as to determine the location of the free surface seepage line, the quantity of seepage through the dam, the pore water pressure distribution and the total head measurements. The results from this program showed acceptable accuracy compared with the experimental results.
2. The K_x/K_y ratio has a significant effect on the location of the free surface line. The free surface is higher by increasing the ratio and it is lower by decreasing the ratio.
3. The quantity of seepage increases as the K_x/K_y ratio increases, therefore, the loss of water through the dam increases.
4. The total seepage quantities of Duhok dam for K_x/K_y ratios (1, 5, 10) are (987.7, 4066.8, 7901.7) m^3/day , respectively, for reservoir water level 613.12 m (a. s. l.).
5. The K_x/K_y ratio has a direct effect on the stability of slopes. Factor of safety decreases with the increase of K_x/K_y ratio, this is due to the increase of pore water pressure and the high free surface seepage line.
6. The slope stability analysis results obtained from the application of a computer program (STABIL2.3) on Duhok dam showed that the dam is safe against the danger of slope failure due to slippage under the operation heads.
7. Results of the present study showed that field piezometer readings of the dam are not accurate probably due to bad installation of piezometers and personal errors.

9. REFERENCES

1. Ali, S., and Fardin, J., “**3D Steady State Seepage Analysis of Embankment Dams**”, *Electronic Journal of Geotechnical Engineering (EJGE)*, Vol. 10, Bandle F, 2005.
2. Karina Dahl, PE, “**Seepage in Earth Embankment Dams**”, Term Project, EC1281A, 2004.
3. Harr, M. E., “**Groundwater and Seepage**”, McGraw-Hill Book Company, New York, U. S. A, 1962.
4. Das, B. M., “**Advanced Soil Mechanics**”, McGraw-Hill Book Company, New York, U. S. A, 1987.
5. Kashef, A. I., “**Groundwater Engineering**”, McGraw-Hill Book Company, New York, U. S. A, 1987.
6. U. S. Army Corps of Engineers (USACE), “**Engineering and Design: Seepage Analysis and Control for Dams**”, EM 1110-2-1901, Washington, U. S. A, 1993.
7. Mishra, G. C., and Singh, A. K., “**Seepage Through a Levee**”, Technical Notes, *International Journal of Geomechanics*, ASCE, Vol. 5, No. 1, PP. 74-79, 2005.
8. Billstein, M., Sevansson, U., and Johansson, N., “**Development and Validation of a Numerical Model of Flow through Embankment Dams - Comparisons with Experimental Data and Analytical Solutions**”, *Transport in Porous Media*, Vol. 35, No. 3, PP. 395-406, 1999.
9. Rao, S. S., “**The Finite Element Method in Engineering**”, Pergamon Press plc, Oxford, Great Britain, 1989.
10. Muhammad, J. R., “**Calculating Seepage Through Earth Dams by Finite Element Method**”, M.Sc. Thesis, College of Engineering, University of Salahaddin, Iraq, 1991.
11. Abo, A. A., “**Calculating Seepage Through Earth Dams by Finite Element Method**”, M.Sc. Thesis, College of Engineering, University of Baghdad, Iraq, 2001.
12. Thieu, N. T. M., Fredlund, M. D., Fredlund, D. G. and Hung, V. Q., “**Seepage Modeling in a Saturated/Unsaturated Soil System**” *International Conference on Management of the Land and Water Resources*, MLWR, Hanoi, Vietnam, Oct. 20-22, 2001.
13. Bardet, J. P., and Tobita, T., “**A Practical Method for Solving Free-Surface Seepage Problems**”, *Computers and Geotechnics*, Vol. 29, PP. 451-475, 2002.
14. Lambe, T. W., and Whitman, R. V., “**Soil Mechanics**”, John Wiley and Sons, Inc., SI Version, New York, U. S. A, 1979.
15. Bishop, A. W., “**The Use of the Slip Circle in the Stability Analysis of Slopes**”, *Geotechnique*, Vol. 5, No. 1, PP. 7-17, 1955.
16. Yaeger, S., “**Slope Stability and Methods of Increasing the Factor of Safety**”, EC1281a, Department of Civil and Environmental Engineering, University of California, 1998.
17. Craig, R. F., “**Soil Mechanics**”, Padstow Ltd., Cornwall, Great Britain, 1990.
18. Wright, S. G., Kulhawy, F., and Duncan, J. M., “**Accuracy of Equilibrium Slope Stability Analysis**”, *Journal of the Soil Mechanics and Foundations Div.*, ASCE, Vol. 99, No. SM10, PP. 783-791, 1973.
19. Martins, F. F., Valente, B. D. S. S., Vieira, C. F. S., and Martins, J. B., “**Slope Stability of Embankments**”, University of Minho, Civil Engineering Department, Portugal, 1999.
20. Hammah, R., Yacoub, T., Corkum, B., and Curran J., “**A Comparison of Finite Element Slope Stability Analysis with Conventional Limit Equilibrium Investigation**”, *Proceedings of the 58th Canadian Geotechnical and 6th Joint IAHCNC and CGS Groundwater Specialty Conferences Saskatoon*, Saskatchewan, Canada, 2005.

21. Ismaeel, Kh. S., "*Seepage and Stability Evaluation of Duhok Dam*", M. Sc. Thesis, College of Engineering, University of Duhok, Iraq, 2006.
22. A Grocomplect Consulting Engineers, (GCE), "*Geotechnical Characteristics of Duhok Dam Embankment Material*", Final Report, Drawing No. 209, 61 Boul Vitosha-Sofia, Bulgaria, 1979.
23. Groundwater Modeling System (GMS), "*SEEP2D Primer*", Brigham Young University, Great Britain, 1998.
24. Krasil'nikov, N. A., "*The Practice of Developing Criteria of the Safe State of an Earth Dam with Respect to Slope Stability*", *Hydrotechnical Construction*, Vol. 27, No. 12, PP. 732-737, 1993.

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